MON 22 1982

CUYAHOGA COUNTY 1 CUY-480-0.00

GEOLOGY OF THE SITE

THE STRUCTURE SITE IS LOCATED ON THE GLACIATED LAKE PLAIN REGION, IN AN AREA WHERE MODERATELY DEEP GLACIAL DRIFT AND LACUSTRINE MATERIALS OVERLIE SANDSTONE BEDROCK, OF DEVONIAN AGE.

EXPLORATION

THE EXPLORATION CONSISTED OF TWO DRIVE SAMPLE-CORE BORINGS AND TEN DRIVE ROD PENETRATION TESTS, MADE BETWEEN MARCH 11 AND 13, 1969.

INVESTIGATIONAL FINDINGS

BORINGS DISCLOSED VERY STIFF TO HARD CLAYS AND VERY DENSE SILTS AND SOME COBBLES 'TO BEDROCK SURFACE, ENCOUNTERED AT 29-FOOT DEPTHS, ELEVATIONS 755 AND 754 FEET. THE BORINGS WERE TERMINATED AT 40-FOOT DEPTHS, ELEVATION 744 FEET, AFTER PENE-TRATING 11 FEET OF BEDROCK.

THE ROD SOUNDINGS ENCOUNTERED RAPID INCREASE IN PENETRATION RESISTANCE WITH IN-CREASE IN DEPTH, AND WERE TERMINATED DUE TO REFUSAL TO PENETRATION AT 11 TO 14-FOOT DEPTHS, ELEVATIONS 772 TO 769 FEET, CONSIDERED TO BE UPON ENCOUNTER WITH THE VERY DENSE SILTS REVEALED BY THE BORINGS.

Date Started 3-12-69

743.6

NO FREE WATER WAS ENCOUNTERED IN THE ROD SOUNDING HOLES.

LEGEND

Auger Boring Location - Plan View.

Core Boring Location - Plan View. Drive Rod Penetration Resistance Sounding Location - Plan View.

Press and / or Drive Sample and / or

Capped Pile

Footing on Pile

Top of Rock

Horizontal Bar on Boring Log Indicates the Depth the Sample Was Taken.

Figures Beside the Boring Log in Profile Indicate the Number of Blows for Standard Penetration Test.

> X = Number of Blows for First 6 inches. Y = Number of Blows for Second 6 inches.

Drive Rod Penetration Resistance Sounding Log - Profile

Casing

Resistance "R" < 10,000 lbs.

Resistance "R" > 10,000 lbs.

Indicates Final Measurement of Penetration, in Inches.

Indicates Free Water Elevation.

Indicates Static Water Elevation.

SYMBOLS OF ROCK TYPES

Weathered Siltstone, Mudstone, or Claystone

Siltstone, Mudstone, or Claystone

Weathered Shale

Weathered Sandstone

Sandstone

Leached Dolomite

Dolomite

Leached Limestone

Limestone

GENERAL INFORMATION

Drive Rod Penetration Sounding Tests

Drive rod penetration resistance tests constitute driving a 1.315-inch diameter steel rod, with a 45° cone point, into the ground, using a 122-pound drop-hammer with a free fall of five feet. At one or two-foot depth intervals, a measurement is taken to determine the amount of penetration achieved in three hammer drops. This reading is converted to an empirical value for capacity "R", in thousands of pounds (which is a measure of both the point resistance and frictional resistance on the rod), by using charts prepared by the Ohio Department of Highways, Bureau of Bridges, on the basis of correlation study of rod penetration with past performance of pile driving. For interpretation, a graph is prepared by plotting the value "R" against the depth at which the reading was taken, and connecting the plotted points. The curve so obtained reflects the density of subsurface materials in a manner that can be readily compared with data from similar tests at other locations on the structure site. From this comparison, the overall uniformity of subsurface condition may be evaluated.

Drive Sample Borings - Drive-Press Sample Borings

Drive sample borings are made by means of a rotary-type drill rig, employing a 2" O.D., 1-3/8" I.D. sampler, at 2-1/2 and / or 5-foot depth intervals, driven by means of a 140 pound drop-hammer with a free fall of 30 inches. The number of blows required to drive the sampler 12 inches is considered the standard penetration test.

Drive-press sample borings are made by means of a rotary-type drill rig, employing a 2" O.D., 1-3/8" I.D. drive sampler, and 3" O.D. thin-wall press sampler. The press sampler is advanced by continuous uniform pressure, applied by the drill rig.

The boring log sheets show a graphic plot of the information obtained, including depth and elevation of the sample, number of blows for the standard penetration tests in two 6-inch increments, depth of press samples, field sample number, sample description - based on lab oratory tests and the Casagrande AC classification system-and gradation, plasticity, and moisture content determinations. Results of strength and consolidation testing, if performed, appear on separate enclosures.

At depths where materials are bouldery or gravelly to the extent that the sampler can not be driven, a wash sample is procured for visual classification, in order to determine the general character of the material. These samples are not considered sufficiently representative to warrant laboratory testing.

Particle Size Definitions

Date Completed 3-13-69
Boring No ______B-5 Dia 3 1/2" Station & Offset 108+60, 43' Lt. (Rear Pier) Surface Elev 783.61 Physical Characteristics Description 778.6 11/12 Brown Silt and Clay 1 0 2 11 25 62 36 12 32 A-6a 776.1 12/15 Brown Silt and Clay 2 0 2 13 26 59 37 12 34 A-6a 773.6 19/24 Gray Gravelly Sandy Clay 3 | 15 | 9 | 10 | 22 | 44 | 30 | 11 | 18 | A-6a 771.1 Gray Gravelly Sandy Clay 4 | 15 | 7 | 11 | 25 | 42 | 30 | 12 | 16 | A-6a 768.6 16 23/27 Gray Sandy Silt 5 | 11 | 8 | 10 | 32 | 39 | 23 | 6 | 10 | A-48 766.1 Gray Sandy Silt 6 9 8 11 31 41 21 6 12 A-48 763.6 Gray Silty Sand 7 | 13 | 9 | 43 | -35 | - | NP | NP | 7 | A-3a 758.6 Gray Gravelly Sandy Silt TOP OF ROCK 754.1 753.6 30 Sandstone (Driller's Description) Sandstone, gray, medium-firm, friable, fine-grained, micaceous in part, 34 thin-bedded. Core Loss 3%. 5.0 0.0

BOTTOM OF BORING **Refusal

LOG OF BORING

	Date Started 3-11-69 Date Completed 3-11-69					Water Elev.										
and a second second second second	Во	oring No.		3-16	Station & Offset 110+50, 25' Rt. (Forward Pier	r)	Surface Elev 784.4'									
Elev.	Depth	Std. Pen. (N)	Rec.	Loss ft.	Description	Sample	T	***************************************		rsical Characteristics						
784.4	٥	1				No.	% Aga	c.s	% FS	% Silt	% Clay	LL	PI.	wc	SHTL Class	
	_2					1								-		
779.4		/														
776.9	-6	11/19			Brown Sandy Clay	1	9	8	12	23	48	35	13	27	A-6a	
110.9	8	12/22			Brown Sandy Clay	2										
774.4	Ю	23/24			,	& 	_	-		-	-	33`	13	16	Vieus]	
771.9	12	~>/ ~4			Gray Sandy Clay	3	13	10	11	26	40	29	11	17	A-6a	
	14	20/30			Gray Sandy Clay	4	8	8	12	26	46	29	11	14	. 4-	
769.4	16	50*	·		Gray Sandy Gravelly Silt	·					·.	~7		3.4	A-6a	
766.9	18	(0.71)			and panel distally 2110	5	26	7	9	26	32	25	7	12	A-4a	
761 1		50/*			Gray Sandy Silt	.6	8	7	11	37	37	24	9	10	A-48	
764.4	20	30/*			Gray Clayey Silt with Cobbles	7						.				
761.9	22					•	-	-	-	-	-	21	7	13	Visual	
759.4	24				No Sample Recovered - Cobbles(Driller's Des.)		Ā	I		s	U		A	L		
× / • 4	26	50*/			No Sample Recovered (Driller's Description)		v			_						
	2.8	(0.31)	0.5	4.5	TOP OF ROCK		٧	1		S	I	,	A	L	,	
54.9	30		•	-	101 OF ROOM											
	32								•							
			4.7	0.3							,	-	1 + +			
	34				Sandstone, gray, medium-firm, friable, fine-graine	d, mic	acec	us :	in p	ert.			.1	•		
	36				thin-bedded. Core Loss 2%.	•			***	· ,			S			
EMONE)	36		5.0	0.0							-					

BOTTOM OF BORING *Refusel

NOTE: Information shown by this subsurface investigation was obtained solely for the use in establishing design controls for the project. The State of Ohio does no luarantee, the accuracy of this data and it is not to be construed as a part of the plans governing construction of the project.

OHIO DEPARTMENT OF HIGHWAYS TESTING LABORATORY 1620 WEST BROAD STREET, COLUMBUS, OHIO 43223

STRUCTURE FOUNDATION INVESTIGATION BRIDGE NO. CUY-480-0150 UNDER STEARNS ROAD

CUY-480-0.00

REVIEWED BY R D R