TE - 118 600 - 4 - 66

OROLOGY OF THE SITES
THE STRUCTURE SITES ARE LOCATED ON THE GLACIATED, MOBERATELY DISSECTED MISSISSIPPI

VALLEY PLAIN, IN AN AREA WHERE SHALLOW GLACIAL DERIVED SOILS OVERLIE SHALE BETROOK. OF MISSISSIPPIAN AGE. ELFLORATION
THE EXPLORATION CONSISTED OF TWO DRIVE SAMPLE-CORE BORINGS AND SIXTREN DRIVE ROD

PENETRATION TESTS, MADE BETWEEN SEPTEMBER 23 AND 30, 1968.

INVESTIGATIONAL FINDINGS BORINGS DISCLOSED STIFF TO VERY STIFF CLAYS OVERLYING BEDROCK SURFACE, ENCOUNTERED AT 10-FOOT DEPTHS, ELEVATIONS 905 AND 896 FEST. THE BORINGS WERE TERMINATED AT 25-FOOT DEPTHS, ELEVATIONS 690 AND 881 FEET, AFTER PENETRATING 15 PERT OF BEDROCK.

THE ROD SOUNDINGS ENCOUNTERED RAPID INGREASE IN PENETRATION RESISTANCE WITH INCREASE IN DEPTH, AND WERE TERMINATED DUE TO REPUSAL OR HEAR-REPUSAL TO PENETRATION AT 7 TO 13-FOOT DEPTHS, ELEVATIONS 909 TO 893 FRET, CONSIDERED TO HE ON BEDEOCK SURFACE, AS REVEALED BY THE BORINGS.

NO FREE WATER WAS ENCOUNTERED IN ANY OF THE ROD SOUNDING HOLES.

IF IT IS THE INTENTION TO FOUND SUBSTRUCTURE UNITS ON BEDROCK, IT IS CONSIDERED AD-VISABLE THAT THE OPEN EXCAVATIONS BE INSPECTED IN THE FIELD, IN ORDER TO INSURE THAT THE EXCAVATIONS HAVE BEEN EXTENDED TO ROCK THROUGHOUT THE ENTIRE POUNDING AREA. IT IS FURTHER SUGGESTED THAT THE AREA OF THE FOOTING CONTACT NOT BE SUBJECTED TO PRO-LONGED ATMOSPHERIC EXPOSURE, AND THAT THE EXCAVATIONS BE WELL DRAINED AT ALL TIMES.

UNCONFINED COMPRESSION TESTS ON SIMILAR SHALE BEDROCK INDICATE A CRUSHING STRENGTH ON THE ORDER OF 100 TONS PER SQUARE FOOT.

15	a	EN	n	
	Y		~	
	7. (7)		_	

Auger Boring Location - Plan View.

Press and / or Drive Sample and / or

Core Boring Location - Plan View.

Drive Rod Penetration Resistance

Sounding Location - Plan View.

Top of Rock

Weathered Indurated Clay

Indurated Clay

Weathered Shale

H	Horizontal Bar on Boring Log Indicates the Depth the Sample Was Taken.
X/Y	Figures Beside the Boring Log in Profile Indicate the Number of Blows for Standar

Y = Number of Blows for Second 6 inches.

X = Number of Blows for First 6 inches.

Drive Rod Penetration Resistance Sounding Log - Profile

Resistance "R" < 10,000 lbs.

Resistance "R" > 10,000 lbs.

Indicates Final Measurement of Penetration, in Inches.

Indicates Free Water Elevation.

Indicates Static Water Elevation.

SYMBOLS OF ROCK TYPES

	Weathered Sandstone

Sandstone

Leached Dolomite

Leached Limestone

Limestone

Dolomite

Drive Rod Penetration Sounding Tests

Drive rod penetration resistance tests constitute driving a 1.315-inch diameter steel rod, with a 45° cone point, into the ground, using a 122-pound drop-hammer with a free fall of five feet. At one or two-foot depth intervals, a measurement is taken to determine the amount of penetration achieved in three hammer drops. This reading is converted to an empirical value for capacity "R", in thousands of pounds (which is a measure of both the point resistance and frictional resistance on the rod), by using charts prepared by the Ohio Department of Highways, Bureau of Bridges, on the basis of correlation study of rod penetration with past performance of pile driving. For interpretation, a graph is prepared by plotting the value "R" against the depth at which the reading was taken, and connecting the plotted points. The curve so obtained reflects the density of subsurface materials in a manner that can be readily compared with data from similar tests at other locations on the structure site. From this comparison, the overall uniformity of subsurface condition may be evaluated.

GENERAL INFORMATION

Drive Sample Borings - Drive-Press Sample Borings

Drive sample borings are made by means of a rotary-type drift rig, employing a 2" O.D., 1-3/8" I.D. sampler, at 2-1/2 and / or 5-foot depth intervals, driven by means of a 140 pound drop-hammer with a free fall of 30 inches. The number of blows required to drive the sampler 12 inches is considered the standard penetration test.

Drive-press sample borings are made by means of a rotary-type drill rig, employing a 2" O.D., 1-3/8" I.D. drive sampler, and 3" O.D. thin-wall press sampler. The press sampler is advanced by continuous uniform pressure, applied by the drill rig.

The boring log sheets show a graphic plot of the information obtained, including depth and elevation of the sample, number of blows for the standard penetration tests in two 6-inch increments, depth of press samples, field sample number, sample description - based on lab oratory tests and the Casagrande AC classification system-and gradation, plasticity, and maisture content determinations. Results of strength and consolidation testing, if performed, appear on separate enclosures.

At depths where materials are bouldery or gravelly to the extent that the sampler can not be driven, a wash sample is procured for visual classification, in order to determine the general character of the material. These samples are not considered sufficiently representative to warrant laboratory testing.

Particle Size Definitions

Boulders | Cobbles | Gravel | Coarse Sand | Fine Sand | Silt

:		Da Da	te Startei ite Compl	eted_	<u>}-26-6</u>	Sampler Type SS Dia 1.3/8** Casing: Length Dia				-		<i>.</i>				
•			ring No.		3-23	Station & Offset 1120+79, 81' Rt. (Forward F	ier)		S	urfa	ce E	lev9	06.2	1		•
Ek	ev. C)epth	Std. Pen. (N)	Rec.	Loss	Description	Sample			·			octeri	stics		SHTL
900	6.2	0					No.	₩ Agg	c's	F.S.	Silt	% Clay	L.L.	P1.	W.C.	
e a de la composition della co	-	2			,											
		4														
901	1.2	6	5/9			Brownish-Gray Gravelly Clay	1	22	3	9	34	32	32	14	19	A-6a
	·	8														
896	5.2	Ю				TOP OF WEATHERED ROCK										
	3.2		50# (0.9°)			Brownish-Gray Weathered Clay Shale	2	24	0	1	41	34	35	12	12	Visus
	-	14		1.9	2.9	TOP OF ROCK										
4 (4) %		16														
	-	<u> 18</u>	~~~	á.1	0.9			4 11	a .	_						
i de la como		20				Shale, dark-gray, weathered at the top, clayey i hard toward bottom of unit. Core Loss St.	n part,	sil	ty s	nd		,				
		22		E 0			,					,				

Refusel

LOG OF BORING

NOTE: Information shown by this subsurface investigation was obtained solely for the use in establishing design controls for the project. The State of Ohio does not guarantee, the accuracy of this data and it is not to be construed as a part of the plans governing construction of the project.

OHIO DEPARTMENT OF HIGHWAYS TESTING LABORATORY 1620 WEST BROAD STREET, COLUMBUS 23, OHIO

STRUCTURE FOUNDATION INVESTIGATION BRIDGE NO. CUY-480-2071 L/R OVER GRANGER ROAD

CHECKED BY REVIEWED BY 10/30/68 L.N.L. R.D.R.

	Во	ite Compli ring No	eted_	1-26-6 3-2	Casing; Length Dia	tment)		S	urfac	e El	ev. <u>9</u>	15.4	<u>+-</u>		
Elev.		Std. Pen.			Description	Sample	Sample Physical Characteristics								SHT
915.4	0	LINI	11.			No.	% Agg	c.s.	% F.S.	% Silit	% Clay	L.L.	Pt.	W.C.	Clas
	2														
910.4	4		•			1		,							
	6	7/11			Brownish-Gray Silt and Clay	1	Ō	3	3	40	54	39	14	19	A-62
	8														
905.4	₁₀ -				TOP OF WEATHERED ROCK										
704.8		50° (0.6°)			Brownish-Gray Vestbered Clay Shale	Z	43	2	3	21	31	36	Ш	14	VLat
San	12	(0.81)			TOP OF BOCK				,	_					
900.4	14		1.9	2.5	Clay Stone, weathered, gray-brown, soft, moist,	shely in	par	4.	COTY) LO	6 4 7	35.			
700.4	<u> </u> 6							·	·····	······			,	(,	
	-		ــــــــــــــــــــــــــــــــــــــ			•			•						
	18		1.5	0.5											
	20				Shale, dark-gray, very platy and finalle, clayey	-		-	Yan-	- 1986					