٦

849.6

847.1

844.6

842.1

839.6

834.6

832.1

827.1

822.1

40

46 50* (0.7')

GROLOGY OF THE SITE

PENETRATION TEST.

Brown and Gray Sandy Gravelly Silt

Brown Silty Sandy Gravel with Boulders

Brown Silty Sandy Gravel with Boulders

Brownish-Gray Gravelly Sandy Silt

Brown Sandy Silt

Brown Silty Sand

Brown Sandy Silt

Gray Sandy Silt

Gray Silty Sand

Gray Gravelly Sand

Gray Silty Sand

Gray Silty Sandy Gravel

THE PENNSYLVANIAN AGE.

TION TESTS, MADE BETWEEN APRIL 17 AND 25, 1968.

TE - 118 600 - 4 - 66

LEGEND

Horizontal Bar on Boring Log Indicates Auger Boring Location - Plan View. the Depth the Sample Was Taken. Press and / or Drive Sample and / or

Figures Beside the Boring Log in Profile Indicate the Number of Blows for Standard Penetration Test.

Casing

X = Number of Blows for First 6 inches. Y = Number of Blows for Second 6 inches.

Drive Rod Penetration Resistance Sounding Log - Profile

Resistance "R" < 10,000 lbs.

Resistance "R" > 10,000 lbs.

Indicates Final Measurement of Penetration, in Inches.

Indicates Free Water Elevation.

Indicates Static Water Elevation.

Weathered Sandstone

SYMBOLS OF ROCK TYPES

Core Boring Location - Plan View.

Drive Rod Penetration Resistance

Sounding Location - Plan View.

Capped Pile

Top of Rock

Weathered Indurated Clay

Indurated Clay

Weathered Shale

Shale

Boulders

Sandstone

Leached Dolomite

 Ξ Dolomite

Leached Limestone

LOG OF BORING

Surface Flow 845.31

Boring No. B-15 Station & Offset 1157+94, 73' Rt. (Forward								Pier) Surface Elev. 845.31									
Elev.	Depth	Std. Pen.	Rec. Loss ft. ft.	Description			Sample					Characteristic			CS	SHTL	
845.3	0	IN)	<u> </u>			· · · · · · · · · · · · · · · · · · ·		No.	% Agg.	C.S.	F.S.	Silt	Cicy	L.L	Pł.	W.C.	Class.
							}										
	2						,										
840.3	4		·	•													
040.5	6	1/3	Brown Silty Sa	undy Gravel				1	48	15	19	-18	_	NP	NP	14	A-1-t
	_			•													
								-					ŀ				
835.3	<u> </u>	10/12	Brown Silty Se	andy Gravel				2	52	16	16	-10	-	NP	NP	12	A-1-t
832.8	12		•	•													
6)2.0	14	13/20	Brown Silt					3	0	1	1	84	14	NP	NP	24	A-4b
830.3	16	12/20	Brown Sandy Si	(14				4	11	9	11	45	11	NP	NP	21	A-4b
Ann A	19	12/20	progn bandy 5:	LAC				44		~		0,					-
827.8	_10_	18/20	Brown Sandy Si	ilt.				5	0	1	21	66	12	NP	NP	20	A-4b
825.3	20										2		-	1770	MD	200	
	22	16/22	Brown Sandy S	nt.		,		6	0	U	36	22	12	NP	NP	20	A-4b
822.8		17/21	Brown Sandy S	!7+				7	0	0	23	64	13	NP	NP	24	A-4b
820.3	24	1	Diown Sandy C.	144				•									
	26	19/30	Brown Silt					8	0	0	9	80	11	NP	NP	24	A-4b
817.8	28			*										1770	3177		4 43
dar a	30	19/24	Brown Silt					9	0	0	14	75	11	NP	NP	24	A-4b
815.3		15/22	Gray Silt					10	0	0	- 5	83	12	NP	NP	23	4 -4b
	32	•															
# 20.0	34	1												ľ			
810.3	36	19/29	Gray Sandy Si	lt				11	0	1	45	46	8	NP	NP	19	A-48
	36																<u>.</u> . :
805.3	40	20/29	Gray Silt		* *						-	-		-	100	-	
	42	20/29	Gray Sile					12	0	0	111	. 78	n	MP.	MP	19	1-43
	44	1	<u>;</u>				•	:							1		
800.3			<u> </u>	1/ 2 .		BOTTOM OF E	CORINC	4.0						-	-		
799.3	46_	21/27	Gray Sandy Sil	. <u>.</u>			1	13	10	1	137	53	19	NP	NP	18	1-4t

GENERAL INFORMATION

Drive Rod Penetration Sounding Tests

Drive rod penetration resistance tests constitute driving a 1.315-inch diameter steel rod, with a 45° cone point, into the ground, using a 122-pound drop-hammer with a free fall of five feet. At one or two-foot depth intervals, a measurement is taken to determine the amount of penetration achieved in three hammer drops. This reading is converted to an empirical value for capacity "R", in thousands of pounds (which is a measure of both the point resistance and frictional resistance on the rod), by using charts prepared by the Ohio Department of Highways, Bureau of Bridges, on the basis of correlation study of rod penetration with past performance of pile driving. For interpretation, a graph is prepared by plotting the value "R" against the depth at which the reading was taken, and connecting the plotted points. The curve so obtained reflects the density of subsurface materials in a manner that can be readily compared with data from similar tests at other locations on the structure site. From this comparison, the overall uniformity of subsurface condition may be evaluated.

Drive Sample Borings - Drive-Press Sample Borings

Drive sample borings are made by means of a rotary-type drill rig, employing a 2" O.D., 1-3/8" 1.D. sampler, at 2-1/2 and / or 5-foot depth intervals, driven by means of a 140 pound drop-hammer with a free fall of 30 inches. The number of blows required to drive the sampler 12 inches is considered the standard penetration test.

Drive-press sample borings are made by means of a rotary-type drill rig, employing a 2" O.D., 1-3/8" 1.D. drive sampler, and 3" O.D. thin-wall press sampler. The press sampler is advanced by continuous uniform pressure, applied by the drill rig.

The boring log sheets show a graphic plot of the information obtained, including depth and elevation of the sample, number of blows for the standard penetration tests in two 6-inch increments, depth of press samples, field sample number, sample description - based on lab oratory tests and the Casagrande AC classification system-and gradation, plasticity, and moisture content determinations. Results of strength and consolidation testing, if performed, appear on separate enclosures.

At depths where materials are bouldery or gravelly to the extent that the sampler can not be driven, a wash sample is procured for visual classification, in order to determine the general character of the material. These samples are not considered sufficiently representative to warrant laboratory testing.

Particle Size Definitions No. 40 sieve No. 200 sieve

> NOTE: Information shown by this subsurface investigation was obtained solely for the use in establishing design controls for the project. The State of Ohio does not guarantee the accuracy of this data and it is not to be construed as a part of the plans governing construction of the project.

OHIO DEPARTMENT OF HIGHWAYS TESTING LABORATORY 1620 WEST BROAD STREET, COLUMBUS 23, OHIO

STRUCTURE FOUNDATION INVESTIGATION BRIDGE NO. CUY -80-2140 OVER PENNA RR, NSW RR. AND MILL CREEK

CHECKED BY LNL.

SEC.

REVIEWED BY

5/15/6

R.D.R.

FREE WATER WAS OBSERVED IN ROD SOUNDING HOLES NUMBERS 15 AND 17 AT ELEVATIONS 836 TR AND 834 FEET. NO TEST PENETRATED TO BEDROCK SURFACE. Station & Offset 1154+94, 73' Lt. (2nd Pier) Depth Std. Pen. Rec. Loss ft. ft. No. Agg. C.S. F.S. Stift Clay L.L. P.I. W.C. Class. 1 10 6 13 35 36 28 8 17 A-4a Brown Sandy Silt

BOTTOM OF BORING

2 | 33 | 11 | 16 | 16 | 24 | 29 | 9 | 23 | A-4a

3 0 19 24 22 35 27 10 25 A-42

4 | 48 | 6 | 13 | 14 | 19 | 29 | 9 | 18 | A-2-4

5 12 24 33 19 12 NP NP 20 A-3a

7 | 58 | 13 | 12 -17 - | NP | NP | 14 | A-1-1

8 0 1 21 68 10 NP NP 22 A-4b

9 | 19 | 7 | 19 | 32 | 23 | 20 | 5 | 16 | A-4a

10 0 1 48 42 9 NP NP 22 A-4a

11 0 2 84 -14 - NP NP 18 A-3a

12 33 27 31 - 9 - MP MP 9 A-1-

3 5 77 -15 - NP NP 18

6 V I S U A L | - | - | 12 |

THE STRUCTURE SITE IS LOCATED ON THE BURIED VALLEY OF MILL CREEK, IN AN AREA WHERE

LACUSTRINE, DEEP ALLUVIUM, AND GLACIAL-DERIVED SOILS OVERLIE SANDSTONE BEDROCK, OF

EXPLORATION
THE EXPLORATION CONSISTED OF TWO DRIVE SAMPLE BORINGS AND NINE DRIVE ROD PENETRA-

INVESTIGATIONAL FINDINGS
BORINGS ENCOUNTERED UNSTRATIFIED LOOSE TO VERY DENSE SANDY SILTS AND GRAVELS AND

SANDS WITH SCATTERED INTERVALS OF BOULDERS. THE BORINGS WERE TERMINATED AT A6

AND 50-FOOT DEPTHS, BLEVATIONS 812 AND 799 FEET, AFTER PENETRATING IN EXCESS OF 25 FEET OF MATERIAL REQUIRING IN EXCESS OF 30 BLOWS PER FOOT IN THE STANDARD

TO 833 FEET, CONSIDERED TO BE ON GRAVELS, AS REVEALED BY THE BORINGS.

ROD SOUNDINGS ENCOUNTERED GENERALLY GRADUAL RESISTANCE TO PENETRATION WITH INCREASE IN DEPTH WITH ERRATIC INTERVALS, INDICATIVE OF GRAVELLY ZONES, AND WERE TERMINATED UPON ENCOUNTER WITH REFUSAL TO PENETRATION AT 16 TO 38-FOOT DEPTHS, ELEVATIONS 844